203-3.4.4 Rubberized Polymer Modified Emulsion (RPME).

203-4.4.1 General. RPME shall consist of a mixture of emulsified asphalt, crumb rubber, and polymer. RMPE shall contain between 0.55 pounds per gallon (66 grams/liter) and 0.65 pounds per gallon (78 grams/liter) of crumb rubber. Latex shall constitute a minimum of 2 percent by weight of RPME.

Crumb rubber shall:

- a) be granulated scrap tire rubber free from fabric, wires, and other contaminants
- b) be dry and free flowing;
- c) have a specific gravity between 1.15 and 1.20; and
- d) have a gradation of 100 percent passing the No. 16 (1.18mm) sieve, 95 percent passing the No. 20 (850 μ m) sieve, and a maximum of 1 percent passing the No. 200 (75 μ m) sieve.

Calcium carbonate or talc may be added to a maximum of 4 percent by dry weight of crumb rubber to prevent the rubber particles from sticking together.

203-3.4.4.2 Composition. RPME shall conform to the requirements to shown in Tale 203-3.4.4.2.

TABLE 203-3.4.4.2

Tests	ASTM	Requirements		
Tests	Test Method	Min.	Max.	
Viscosity, 77°F (25°C), Brookfield, Model RVT #6 Spindle @ 10 RPM (Centipoise) @ 60 sec.	D 2196	4,000	12,000	
Residue by Evaporation % (including fillers)	D 6934	50	55	
Sieve Test (% retained on No. 20 (850μm) sieve)	D 6933		2.0^{1}	
Penetration of Residue, 77°F (25°C), 0.1mm	D 5	15	30	
Solubility of Residue	D 2042	75		
Weight lbs/gallons (g/L) 77°F± 1°F (25°C ± 5°C)	D 1475	8.33 lbs/gal (1000g/L)	8.75 lbs/gal (1050g/L)	
Asphalt Content ²		40		

^{1.} Sieve test of original emulsion is 0.10 max.

203-3.4.5 Polymer Modified Emulsion (PME). PME shall conform to the requirements shown in Table 203-3.4.5 (A). Quick-set PME shall conform to the requirements shown in Table 203-3.4.5 (B).

PME shall be sample in accordance AASHTO T 40.

^{2.} Asphalt Content shall be determined by multiplying Residue by Evaporation by Solubility of Residue.

TABLE 203-3.4.5 (A) POLYMER MIDIFIED EMULSION

		Anionic			Cationic				
		PMF		PMF	RS-2h	PM	RS-2	PMR	RS-2h
Properties	Type Grade	Min.	Max.	Min.	Max.	Min. Max.		Min.	Max.
Test on emulsions:	Test Method		•						
Saybolt Furol Viscosity, @50 °C, SFS1		75	300	75	300	75	300	75	300
Settlement, 5 Days, %			5		5		5		5
Storage Stability. 1 Day, %			1		1		1		1
Sieve Test, %			0.3		0.30		0.3		0.30
Demulsibility, %		60^{2}		60^{2}		60^{2}		60^{2}	
Particle Charge									
Ash Content, %	ASTM D723		0.2		0.2		0.2		0.2
Residue by Evaporation, %	AASHTO T 59	65		65		65		65	
Test on residue from evaporation:	AASHTO T 59								
Penetration, 25°C	AASHTO T 49	100	200	40	90	100	200	40	90
Ductility, 25°C, mm	AASHTO T 51	65		65		65		65	
Elastic Recovery, %	AASHTO T 301	65		65		65		65	
Softening Point °F	AASHTO T 53			135				135	

^{1.} SFS means Saybolt Furol Secons.

TABLE 203-3.4.5 (B) QUICK-SET POLYMER MODIFIED EMULSION (PMCOS-1h)

		Val	lue
Property	Test Method	Min.	Max.
Test on emulsion:			
Saybolt Furol Viscosity @ 25°C, SFS ¹	AASHTO T 59	15	90
Sieve test, %	AASHTO T 59		0.30
Storage stability, 1 day, %	AASHTO T 59		1
Residue by evaporation, %	AASHTO T 59	60	
Particle charge	AASHTO T 59		
Test on residue by evaporation:			
Penetration, 25°C	AASHTO T 49	40	90
Ductility, 25°C, mm	AASHTO T 51	400	
Elastic Recovery, %	AASHTO T 301	50	
Softening Point, °F	AASHTO T 53	135	

^{1.} SFS means Saybolt Furol Secons.

203-3.4.6 Microsurfacing Emulsion (MSE). MSE shall be a quick-set, homogeneous, polymer modified, cationic asphalt emulsion. MSE shall conform to the requirements shown in Table 203-3.4.6.

^{2.} Use 35 ml of 0.02 normality CaCl₂ solutions.

^{3.} Use 35 ml of 0.8% sodium dioctyl sulfosuccinate solution.

^{4.} PME shall contain a minimum of 2.5 percent by weight of residual asphalt.

^{2.} PMCQS-1h shall contain a minimum of 205 percent by weight of residual asphalt

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TABLE 203-3.4.6

Test	Test Method	Requirement
Viscosity @ 25°C, SSF	AASHTO T 59	15-90 sec
Sieve Test, max.	AASHTO T 59	0.30%
Settlement, 5 days, max.	AASHTO T 59	5%
Storage Stability, 1 day, max.	AASHTO T 59	1%
Residue by Evaporation, min.	AASHTO T 59	62%
Test on Residue:		
Test	Test Method	Requirements
Penetration @ 77°F (25°C)	AASHTO T 49	40-90
Softening Point, min.	AASHTO T 53	135°f (57°C)
Elastic Recovery, min.	AASHTO T 301	50

Polymers shall be milled or blended into the asphalt or blended into the emulsifier solution prior to the emulsification process. MSE shall contain a minimum of 3 percent polymer solids based on the weight of residual asphalt.

The 5-Day settlement test may be waiver, provided MSE stored for use on the Work site is used within 36 hours from the time of shipment.

A Certificate of Compliance for each shipment of MSE delivered to the Work site shall be submitted to the Engineer.

203-3.5 Certificate of Compliance. A Certificate of Compliance shall be sent with each load of emulsified asphalt to be delivered to the Work or production site.

Test date for each property specified shall be submitted upon request. The test frequency for each specified property shall bot exceed one year.

203-3.6 Temperature. Emulsified asphalt may be reheated, but at no time after loading shall the temperature be raised above 160°F (70°C). During reheating, emulsified asphalt shall be agitated to prevent localized overheating. Emulsified asphalt shall not be permitted to cool to a temperature less than 40°F (5°C).

203-3.7 Volumetric Measurement. For volumetric quantities, the unit of measurement shall be the U. S. gallon (L) at a temperature of 60°F (15°C)

In converting weight to volume, computations shall be based on the following, for all grades of emulsified asphalt, except RPME:

Gallons per ton = 240 (L/tonne = 1002)

Pounds per gallons = 8.33 (L/tonne = 998)

For RPME:

Gallons per ton = 235 (L/tonne = 962)

Pounds per gallon = 8.5 (1/tonne = 1018)

203-4 MICROSURFACING.

203-4.1 General. Mircosurfacing shall be a mixture of microsurfacing emulsion (MSE), water, set control agents and aggregate.

203-4.2 Materials.

203-4.2.1 Aggregate.

203-4.2.1 General. Aggregate, excluding mineral filler, shall be 100 pernent crushed rock conforming to 200-1.1, 200-1.2 and the requirements shown in Table 203-4.2.1.1

TABLE 203-4.2.1.1

Test	ASTM Test Method	Requirement
San Equivalent	D2419	65 Minimum
Percentage Wear at 500 Revolutions ¹	C131	35% Maximum

^{1.} To be run on material retained on the No. 4 (4.75 mm) sieve graded from the source.

2.3-4.2.1.2 Grading. The combined aggregate gradation shall be determined in accordance with ASTM C136 and conform to the requirements shown in Table 203-5.3.2 for Type II and Type III.

203-4.2.2 Microsurfacing Emulsion (MSE). MSE shall conform to 203-5.4.2.5.

203-4.2.3 Water. Water shall conform to 203-3.2

203-4.2.4 Set Control Agents. Set control agents shall conform to 203-5.4.2.5

203-4.3 Mix Design. The Contractor shall submit a mix design for each aggregate gradation (type) to be used in the Work. Laboratory reports supporting each mix design shall be included with the submittals. The component materials used in the mix design must be the same materials that will be used during microsurfacing application.

The percentages of each individual material proposed in the mix design shall be shown in the laboratory report. Individual materials shall be within the limits shown in Table 203-4.3 (A)

TABLE 203-4.3 (A)

MSE Residual Asphalt	Type II: 5.5% to 10.5% by dry weight of aggregate Type III: 6.5% to 10.5% by dry weight of aggregate
Water and Additives	No Limit
Set Control Agents	0% to 3% by dry weight of aggregate

Microsurfacing mixtures shall conform to the requirements shown in Table 203-4.3 (B)

203-4.3 (R)

203-4.3 (D)								
Test	ISSA Test	Requirements						
	Method							
Wet Cohesion	TB* 139							
@30 Minute (Set), min.		12 kg-cm						
@30 Minute (Traffic), min.		20 kg-cm						
Excess Asphalt, max.	TB* 109	$50 \text{ g/ft}^2 (540 \text{ g/m}^2)$						
Wet Stripping, min.	TB* 114	Pass (90% Minimum)						
Wet Track Abrasion Loss	TB* 100							
6-day Soak, max.		$75 \text{ g/ft}^2 (810 \text{ g/m}^2)$						
Displacement	TB* 147							
Lateral, max.		5%						
Specific Gravity After 1000 Cycles of								
125 lbs (57 kg), Max.		2.10						
Classification Compatibility, min.	TB* 144	(AAA, BAA) 11 Grade Points Minimum						
		· · · · · · · · · · · · · · · · · · ·						

Mix Time @ 77°F(25°C), min.	TB* 113	Controllable to 120 Seconds Minimum
mp.t. m. t. t. t.p. tt. t		

TB* = Technical Bulletin

The laboratory report(s) shall show the results of the test on individual materials and shall compare their values to those required. The report shall clearly show the proportions of aggregate, water (minimum and maximum), set control agents, and MSE residual asphalt content (minimum and maximum) based on the dry weight of aggregate. The laboratory report(s) shall include the quantitative effects of moisture content on the unit weight of aggregate (bulking effect) in accordance with ASTM C29M, The laboratory report shall further show the recommended changes in water and set requirements for 60 seconds of mix time with material heated to 100°F (38°C).

The completed mixture, after addition of water and set control agents, if set control agents are used, shall be such that the microsurfacing mixture has the workability necessary for application. At the expiration of the time allowed for closure of roadways of traffic lanes, the microsurfacing mixture shall be sufficiently cured to support unrestricted traffic.

203-5 SLURRY SEAL.

203-5.1 General. Slurry seal shall be either emulsion-aggregate slurry (EAS) conforming to 203-5.4 or rubberized emulsion-aggregate slurry (REAS) conforming to 203-5.5.

203-5.2 Mix De2sign. The Contractor shall submit a mix design for each combination of emulsified asphalt grade and aggregate gradation to be used in the Work. Each mix design shall conform to ASTM D3910. Laboratory reports supporting each mix design shall be included with the submittals. Laboratory reports shall identify the aggregate source and supplier, emulsified asphalt supplier and all of the test results required to ASTM D3910 except for the cohesion test. When the use of slow-set emulsion is specified, the "set time" test is not required. In addition, each mix design shall include the following:

- a) amount of PME or RPME in gallons (liters) per ton (tonne) of aggregate,
- b) amount of added water in gallons (liter) per ton (tonne) of aggregate,
- c) quantity and type of set control agents,
- d) percent of polymer, if so specified, and the
- e) loose unit weight of aggregate (ASTM C29 with 0.1 cubic feet (3L) bucket).

Mix designs shall be based on the following:

- a) for Type Fine, the content of RPME and water needed to produce a slurry seal with a maximum wet track abrasion test loss of 50 grams per square foot (540 grams per square meter) when tested in accordance with ASTM D3910;
- b) for Type I, the content of emulsified asphalt and water needed to produce a Type I slurry seal with a maximum wet track abrasion test loss of 50 grams per square foot (540 grams per square meter) when tested in accordance with ASTM D3910; or,
- c) for Type II or Type III, the content or emulsified asphalt and water needed to produce a slurry seal with a maximum wet track abrasion test loss of 60 grams per square foot (650 grams per square meter) when tested in accordance with ASTM D3910.

ASTM D3910 shall be modified to include the aggregate retained on the No. 4 (4.75 mm) sieve for Type II and Type III slurry seals. Type III slurry seals shall use the 3/8 inch (9.5 mm) template.

When a mix design is more than 30 Days old, it shall be supplemented with a Certificate of Compliance that states the combined aggregate gradation is within ± 3 percent of the referenced mix design based on a 30-Day moving average or the average of a minimum of 10 of the most current laboratory results, whichever is greater. A mix design shall be reformulated if it is more than one-year-old or whenever that combine aggregate gradation changes from that in the previous submitted mix design by ± 3 percentage points on any sieve size shown in Table 203-5.3.2. If the source of any aggregate or emulsified asphalt is changed, or the mix design or supporting laboratory reports are over one-year-old, a new mix design shall be submitted.

203-5.3 Aggregate.

203-5.3.1 General. Aggregate shall be 100 percent crushed rock conforming to 200-1.1, 200-1.2, and the requirements shown in Table 203-5.3.1.

TABLE 203-5.3.1

Tests	ASTM Test Method	Requirements
Percentage Wear at 500 Revolutions ¹	C131	40% Maximum
San Equivalent	D2419	55 Minimum
Soundness (5 cycles) ¹	C88	15% maximum

^{1.} To be run on the material retained on the No. 4 (4.75mm) sieve graded from the source.

203-5.3.2 Grading. The combined aggregate gradation shall be determined in accordance with ASTM C136 and confirm to the requirements shown in Table 203-5.3.2.

TABLE 203-5.3.2

TABLE 205-5.5.2											
	Percentage Passing Sieves										
	Туре	Type Fine Type I		Typ	e II	Type III					
Sieve Size	Min.	Max.	Min.	Max.	Min.	Max.	Min.	Max.			
3/8 inch (9.5mm)	100		100		100		100				
No. 4 (4.74mm)	100		100		90	100	70	90			
No. 8 (2.36 mm)	95	100	90	100	65	90	45	70			
No. 16 (1.18mm)	75	92	65	90	45	70	28	50			
No. 30 (600μm)	50	75	40	60	30	50	19	34			
No. 50 (300μm)	35	50	25	42	18	26	12	25			
No. 100 (150μm)	15	30	15	30	10	24	7	18			
No. 200 (75μm)	10	20	10	20	5	15	5	15			

203-5.4 Emulsion-Aggregate Slurry (EAS)

203-5.4.1 General. EAS shall be a mixture of polymer modified cationic quick-set emulsified asphalt, aggregate, water, a set control agent. EAS shall be specified by combined aggregate gradation and emulsified asphalt grade, i.e. Type II-PMCQS-1h-EAS. The combined aggregate gradation and emulsified asphalt grade shall be as specified in the Special Provisions or shown on the Plans.

203-5.4.2 Materials.

203-5.4.2.1 Aggregate. Aggregate shall be Type I, Type II, or Type III conforming to 203-5.3.

203-5.4.2.2 Emulsified Asphalt. Emulsified asphalt shall be PMCQS-1hr conforming to the requirements shown in Table 203-3.4.5 (B) unless otherwise specified or shown on the Plans.

The percentage of emulsified asphalt and residual asphalt content shall conform to the requirements shown in Table 203-5.4.2.2.

TABLE 203-5.4.2.2

	Test Method	Type I	Type II	Type III
Emulsified Asphalt %, by weight of dry aggregate		17 - 20	14 - 18	11 - 15
Residual Asphalt	ASTM D6307 ¹	10 min.	7.5 min.	6.5 min.
Content, % by weight of dry	or			
aggregate	CT 3821			

^{1.} Sample size shall be 500g minimum.

203-5.4.2.3 Water. Water shall conform to 203-3.2

203-5.4.2.4 Not Used.

203-5.4.2.5 Set Control Agents. Set control agents shall be either Type II or Type V Portland cement conforming to 201-1.2.1, aluminum sulfate, or other material approved by the Engineer.

203-5.4.3 Mixing. Mixing shall conform to 302-4.

203-5.5 Rubberized Emulsion Aggregate Slurry (REAS).

203-5.5.1 General. REAS shall be a stable mixture of RPME, aggregate, water and Portland cement. REAS shall be specified by combined aggregate gradation, i.e. Type II-REAS. The combined aggregate gradation shall be as specified in the Special Provisions or shown on the Plans.

203-5.5.2 Materials.

203-5.5.2.1 Aggregate. Aggregate shall be Type Fine, Type I, Type II or Type III conforming to 203-5.3 of RPME and residual RPME solids shall conform to the requirements shown in Table 203-5.5.2.2.

203-5.5.2.2 Rubberized Polymer Modified Emulsion. RPME shall conform to 203-3.4.5. The percentage of RPME and residual RPME solids shall conform to the requirements shown in table 203-5.5.2.2.

TABLE 203-5.5.2.2

Test	Test	Type Fine		Type I		Type II		Type III	
	Method	Min	Max	Min	Min	Min	Max	Min	Max
RPME % by weight of	ASTM	61	85	50	57	33	40	28	35
dry aggregate ¹	$D6307^2$ or								
Residual RPME Solids ³	CT 382 ²	31	47	26	31	17	22	15	19

^{1.} Must meet Residual RPME Solids.

^{2.} Sample size shall be 500g minimum.

^{3.} Residual RPME Solids shall be determined by multiplying RPME % (ASTM D6307 or CT 382) by Residue by Evaporation of RPME % (ASTM D6934).

203-5.5.2.3 Water. Water shall conform to 203-3.2.

203-5.5.2.4 Portland Cement. Portland cement may be added to modify the viscosity and curing characteristics accordance with the approved mix design.

Portland cement shall be Type I/II or II/V conforming to 201-1.2.1 and shall not exceed 1.5 percent of the dry weight of the aggregate.

203-5.5.3 Central Plant Mixing.

203-5.5.3.1 General. Mixing at a central mixing plant shall conform to the following requirements:

- a) Component materials conforming to 203-5.5.2 shall be stored separately at the plant.
- b) Aggregate shall neither be stored nor transported in such a way that may cause segregation, degradation, or intermingling of different size aggregates.
- c) Materials shall be proportioned by weight into the mixing tank. Volumetric proportioning will not be permitted.
- d) The mixing tank shall be equipped with scales. The zero tolerance for the tank scales shall be 0.5 percent based on the total batch weight. The scales shall be calibrated and certified on a yearly basis in accordance with 4-1.7 or after every modification or repair.
- e) The mixing tank shall be equipped with a full sweep mixer/agitator with blades or paddles of a sufficient size and number and operated at a speed sufficient to produce a homogeneous mix. Should the blades, paddles, or other parts of the mixer/agitator become worn to such an extent to adversely affect the quality of the mix they shall be promptly replaced. Insufficient mixing or agitation within the mixing tank shall be corrected by either a reduction in the volume of component materials or other adjustments.
- f) Each batch shall be continuously mixed for 3 minutes or until all of the component materials are thoroughly blended, whichever is longer. The mixing time shall begin upon the introduction of the last component material. If the Engineer determines that the mixture is not thoroughly blended, the mixing time shall be increased.
- g) Mixed REAS shall be stored at the central mixing plant site in storage tanks equipped with an agitator if a similar configuration to the agitator in the mixing tank. The agitator shall be capable of continuous operation.

203-5.5.3.2 Transporting. REAS shall be transported from the central mixing plant to the Work site in trucks specifically designed for this purpose equipped with an agitator. REAS shall be continuously agitated during transport.

203-5.5.4 Work Site Mixing. Mixing at the Work site in continuous-flow mixers shall conform to 302-4.

302-4.7 Rubberized Emulsion Aggregate Slurry (REAS).

302-4.7.1 General. REAS shall conform to 203-5.5

- **302-4.7.2 Mixing.** REAS shall be mixed by one of the following methods as specified in the Special Provisions:
 - a) in a continuous flow mixer conforming to 302-4.3 at the Work site or
 - b) at a central mixing plant conforming to 203-5.5.3
- **302-4.7.3 Transporting.** Transporting of REAS mixed at a central mixing plat shall conform to 203-5.5.3.2.
- **302-4.7.4 Work Site Storage.** REAS mixes at a central mixing plat may be stored at the Work site in tanks specifically designed for this purpose and which are equipped with an agitator similar to that in a central mixing plant. The agitator shall be capable of continuous operation.
- **302-4.7.5 Application Temperature.** REAS shall not be applied if either the pavement or the ambient temperature is less than 55°F (13°C) and falling, but may be applied when the pavement and ambient temperatures are both above 50°F (10°C) and rising.

302-4.7.6 Application Rate.

302-4.7.6.1 Continuous-Flow Mixer. The aggregate application rate of REAS mixed on the Work site in a continuous-flow mixer shall conform to the requirements shown in Table 302-4.7.6.1 unless otherwise specified in the Special Provisions or shown on the Plans.

TABLE 302.4.7.6.1

Туре	Application Rate (REAS)	
	Minimum	Maximum
Type Fine REAS	2.8 lbs/yd^2	3.4 lbs/yd ²
	(1.5 kg/m^2)	(1.8 kg/m^2)
Type I REAS	4.4 lbs/yd^2	$5.2 \mathrm{lbs/yd^2}$
	(2.4 kg/m^2)	(2.8 kg/m^2)
Type II REAS	7.5 lbs/yd^2	10.0 lbs/yd ²
	(4.1 kg/m^2)	(5.4 kg/m^2)
Type III REAS	15.7 lbs/yd^2	21.8 lbs/yd ²
	(8.5 kg/m^2)	(14.7 kg/m^2)

302-4.7.6.2 Central Mixing Plant. The application rate of REAS mixed at a central mixing plant shall conform to the requirements shown in Table 302-4.7.6.2 unless otherwise specified in the Special Provisions or shown on the Plans.

TABLE 302-4.7.6.2

Туре	Application Rate (REAS)	
	Minimum	Maximum
Type Fine REAS	$5.0 \text{ lbs/yd}^2 (2.7 \text{ kg/m}^2)$	$6.0 \text{ lbs/yd}^2 (3.2 \text{ kg/m}^2)$
Type I REAS	$7.1 \text{ lbs/yd}^2 (3.8 \text{ kg/m}^2)$	$8.4 \text{ lbs/yd}^2 (4.5 \text{ kg/m}^2)$
Type II REAS	$11.3 \text{ lbs/yd}^2 (6.1 \text{ kg/m}^2)$	$15.0 \text{ lbs/yd}^2 (8.1 \text{ kg/m}^2)$
Type III REAS	22.5 lbs/yd ² (12.2 kg/m ²)	28.1 lbs/yd ² (15.2 kg/m ²)

302-4.7.6.3 Corrective Action. When the application rate is less than the minimum shown in the tables above, the Contractor shall apply additional REAS to the nonconforming areas as necessary to conform to the Specification.

When the application rate exceeds the maximum shown in the tables above, the nonconforming material shall be removed and replaced, or be left in a place at no additional cost to the Agency, as determined by the Engineer.

302-4.8 Spreading and Application. Prior to spreading, the Contractor shall clean the existing pavement unless otherwise specified. Immediately ahead of the spreader truck, the existing pavement shall be pre-wetted by a pressurized water distribution system equipped with a fogtype spray bar capable of completely covering the surface of the pavement.

Slurry seal mixed at the Work site shall be spread by a spreader box attached to a continuous-flow mixer truck conforming to 302-4.3. REAS produced at a central mixing plant shall be spread by a spreader box attached to an agitator truck conforming 203-5.5.3.2.

The spreader box shall be equipped with flexible material in continuous contact with the existing pavement and shall be capable of controlling the rate of application. The spreader box shall have adjustable width and strike-off height, and be capable of controlling and providing uniform spreading.

The maximum speed of the spreader truck shall not exceed 270 feet per minute (80 meters per minute).

REAS mixed at a central mixing plant shall be continuously agitated during spreading.

Hand squeegees and other equipment shall be provided for spreading and spillage removal in areas inaccessible to the spreader box.

Slurry seal shall be applied in such a manner that no ridges remain. Areas in which there is evidence of solidification of the emulsified asphalt, balling or lumping of the aggregates, or uncoated aggregates shall be removed and replaced to the satisfaction of the Engineer.

The Contractor shall prevent slurry seal from being deposited on other than asphalt concrete surfaces and shall remove it from surfaces not designated to be sealed. The method of removal shall be approved by the Engineer.

Where the completed slurry seal surfacing is not uniform in color, the street shall be treated by a method approved by the Engineer to eliminate the color variation.

302-4.9 Field Sampling and Testing.

302-4.9.1 Field Sampling. During the performance of the Work, the Contractor shall provide the Engineer with at least 2 field samples, from separate loads, of mixed slurry seal per mixer per day. WTAT specimens shall be cast and struck off within 60 seconds of obtaining the sample. WTAT specimens shall not be transported until the slurry seal has set as defined by ASTM D3910.

Field samples shall conform to the requirements shown in Table 302-4.9.1.

TABLE 302-4.9.1

Test	ASTM Test	Requirements	
Test	Method	Min. Max.	
Wet Track Abrasion Test, Weight loss, gm/ft ²	D3910 ¹	0 50 (540)	
(gm/m ²) Type Fine Aggregate			
Wet Track Abrasion Test, Weight loss, gm/ft ²	D3910 ¹	0 50 (540)	
(gm/m ²) Type I Aggregate			
Wet Track Abrasion Test, Weight loss, gm/ft ²	D3910 ¹	0 60 (650)	
(gm/m ²) Type II Aggregate			
Wet Track Abrasion Test, Weight loss, gm/ft ²	D3910 ¹	0 60 (650)	
(gm/m ²) Type III Aggregate			
Consistency Test (mm)	D3910 ¹	20 40	
Extraction Test (Calculated Emulsion Content, %)	D6307 ² , CT 382 ²	\pm 1% of mix design for EAS	,
		\pm 3% of mix design for REAS	S
Water Content (% of Dry Slurry)	See Note 3	Type I, II and III EAS < 25	
•		Type Fine and I REAS < 40	
		Type II and III REAS < 31	

- 1. Modified ASTM D3910 to include No. 4 (4.75mm) aggregate or greater and to be performed using field samples. Subsection 6.4.4.7, ASTM D3920 may be modified to use a microwave oven for drying the specimen after the abrasion cycle is complete and the debris washed off.
- 2. Modified ASTM D6307 and California Test Method 382 to allow a minimum of 500 ±50 gram sample.
- 3. Weight a minimum of 500 grams of homogenized mixed slurry into a previously tared quart can with a friction lid. The lid shall be placed on the can to prevent loss of material during transportation. Place the can with the lid off in an oven and dry to constant mass at $220^{\circ}\text{F} \pm 10^{\circ}\text{F}$ ($110^{\circ}\text{C} \pm 5^{\circ}\text{C}$).
- 4. The 3/8 inch (9.5mm) template shall be used.

If the test results fail to meet Specification, the Contractor shall cease spreading slurry seal produced by the nonconforming mixed until the Contractor demonstrates the mixer is producing slurry seal which conforms to the Specification.

302-4.10 Measurement.

- **302-4.10.1 General.** The basis of measurement shall be the weight of materials, in tons (Tonnes), used in the Work, as determined by licensed weight master's certificates. Upon completion of the Work, the Contractor shall submit to the Engineer licensed weightmaster's certificates for materials delivered to the Work site and for excess materials not incorporated into the Work.
- **302-4.10.2 Slurry Seal Mix in Continuous-Flow Mixers.** Slurry seal mixed in continuous-flow mixers shall be measured by each ton (Tonne) of emulsified asphalt and each ton (Tonne) of each type of aggregate used in the Work.
- **302-4.10.3 REAS Mixed at a Central Mixing Plant.** REAS mixed in a central mixing plant shall be measure by each ton (Tonne) used in the Work, including aggregate, RPME, additives and water.

302-4.11 Payment.

- 302-4.11.1 Payment Reduction for Noncompliance.
- **302-4.11.1.1 General.** Payment to the Contractor will be reduced for failure of the field test samples to conform to the WTAT requirements specified in 302-4.9.1.

302-4.11.1.2 Reduction in Payment Based on WTAT. If the average of all WTATs performed per mixer, per day, fails to conform to the requirements specified in 302-4.10.1, the Contractor agrees that payment for the Work represented by the failed tests shall be reduced as shown in Table 302-4.11.1.2 (A) or (B).

TABLE 302-4.11.1.2 (A)

WTAT Loss gm/ft ² (gm/m ²)	Payment Reduction (Percent) Type Fine & Aggregate
0 - 50 (0 - 540)	0
50.1 – 60 (540.1 – 650)	5
60.1 - 70 (650.1 - 750)	15
70.1 - 80 (750.1 - 860)	30
80.1 - 99 (860.1 - 1070)	70
99.1 or greater (1070.0 or greater ¹)	100

^{1.} Slurry seal surfacing with WTAT loss greater than 99.1 gm/m² (1070.0 gm/m²) shall be removed to the satisfaction of the Engineer.

TABLE 302-4.11.1.2 (B)

WTAT Loss gm/ft ² (gm/m ²)	Payment Reduction (Percent) Type II & III Aggregate
0 - 60 (0 - 540)	0
60.1 – 75 (650.1 – 810)	5
75.1 – 80 (810.1 – 860)	15
70.1 – 80 (750.1 – 860)	30
80.1 – 99 (860.1 – 1070)	70
99.1 or greater (1070.0 or greater ¹)	100

^{1.} Slurry seal surfacing with WTAT loss greater than 99.1 gm/m² (1070.0 gm/m²) shall be removed to the satisfaction of the Engineer.

302-4.11.2 Slurry Seal Mixed in Continuous-Flow Mixer. Payment for slurry seal surfacing for slurry seal mixed in continuous-flow mixers shall be at the Contract Unit Price per ton (tonne) for emulsified asphalt and the Contract Unit Price per ton (tonne) for each type of aggregate. No separate payment will be made for calibration, scheduling, public convenience, or traffic control unless otherwise specified.

302-4.11.3 REAS Mixed at a Central Mixing Plant. Payment for slurry seal surfacing for REAS mixed in a central mixing plant shall be at the Contract Unit Price per ton (tonne). No separate payment will be made for calibration, scheduling, public convenience, or traffic control unless otherwise specified.